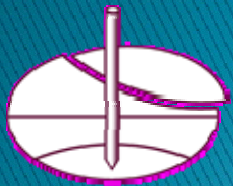


# Pad foundation with vertical central load on dense sand

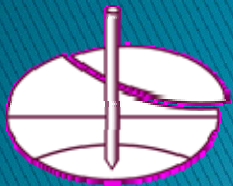
Jose Brito, Cenor, Portugal  
Carsten S. Sorensen, COWI, Denmark



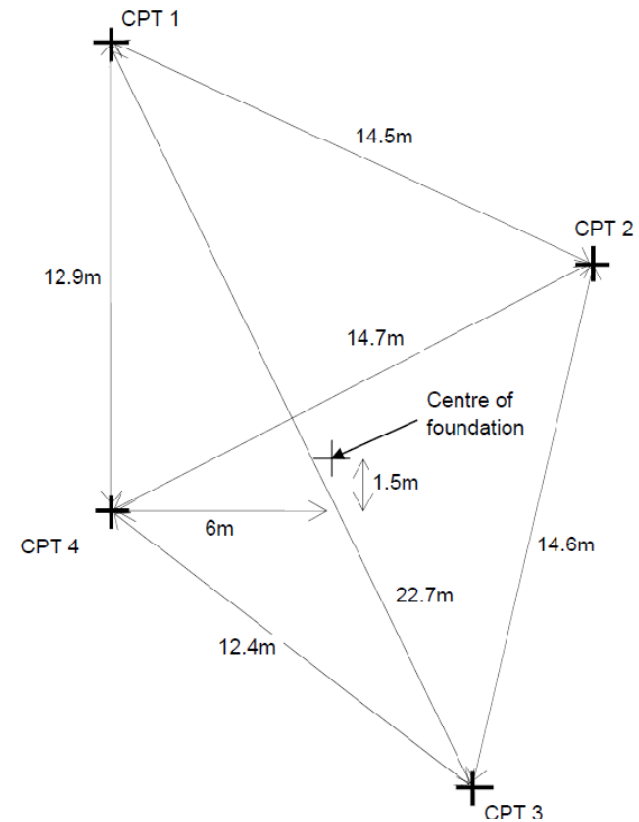
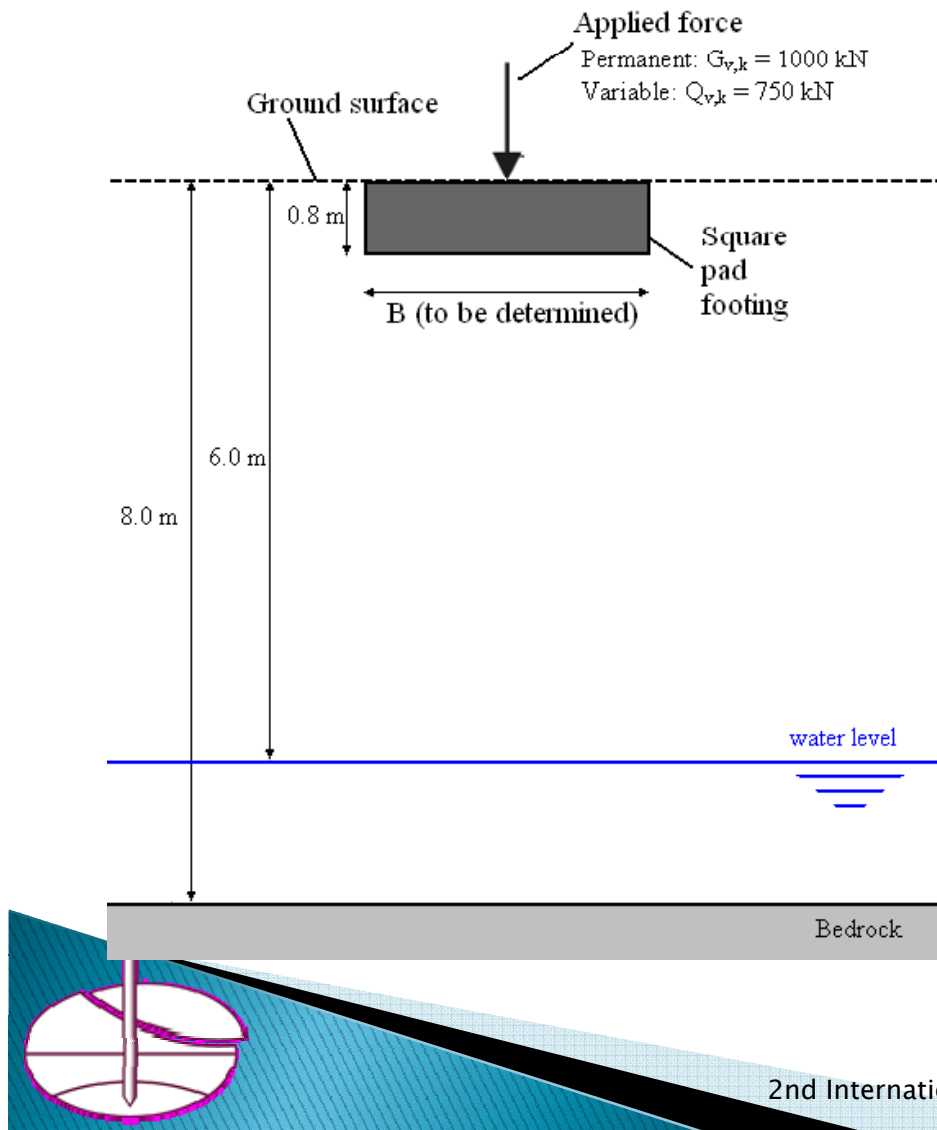
In this example it is asked to design a square pad foundation according to Eurocode 7.

The aim is the evaluation of the foundation width with a maximum allowable settlement of 25 mm.

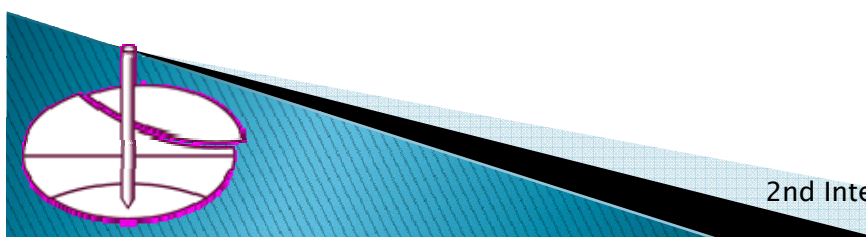
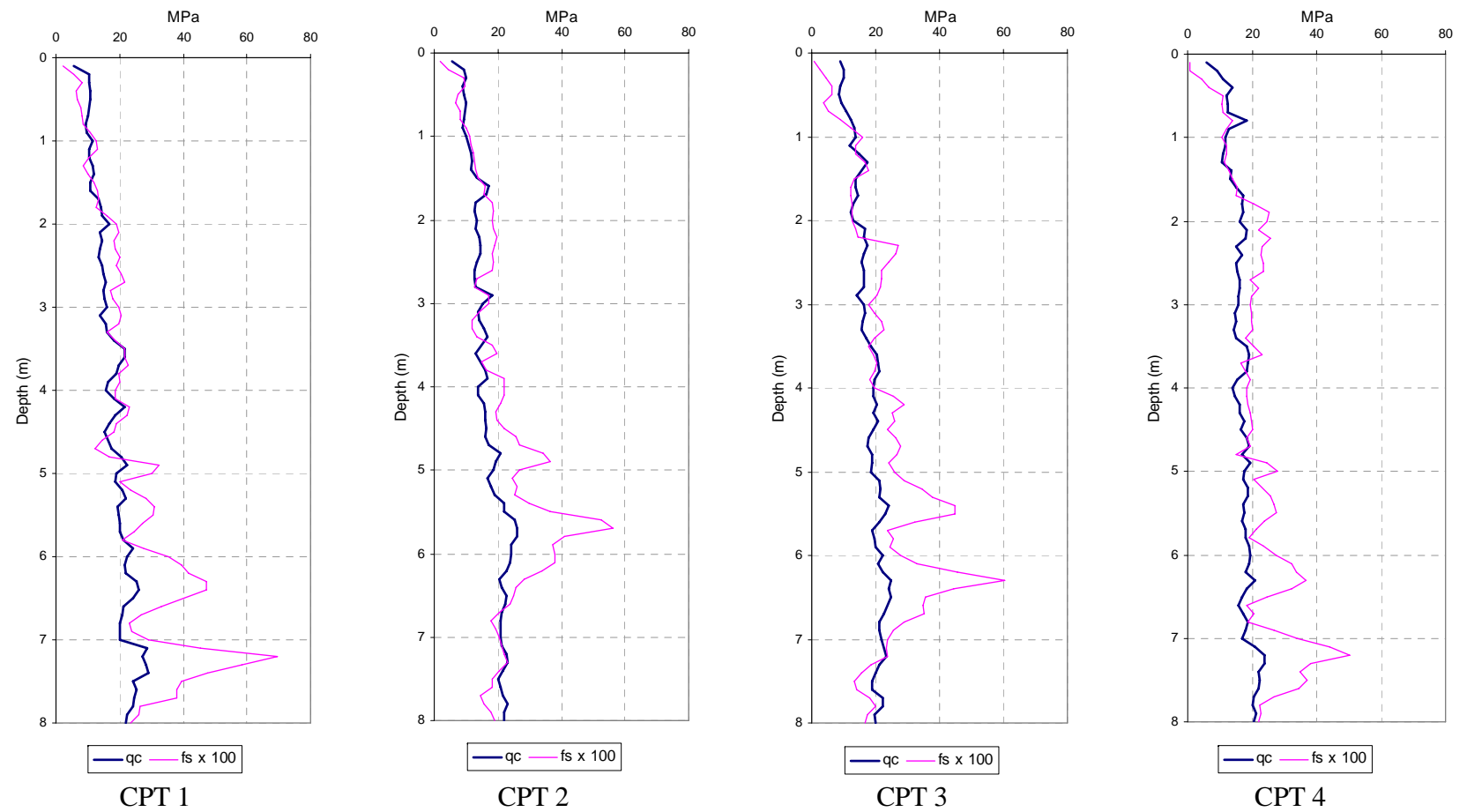
The square pad foundation is made from concrete with a weight density of  $25 \text{ kN/m}^3$  and has an embedment depth of 0.8 m. The ground surface shown can be reliably assumed to be below any topsoil and disturbed ground. The design action is vertical with a permanent load of 1000 kN and a variable load of 750 kN.



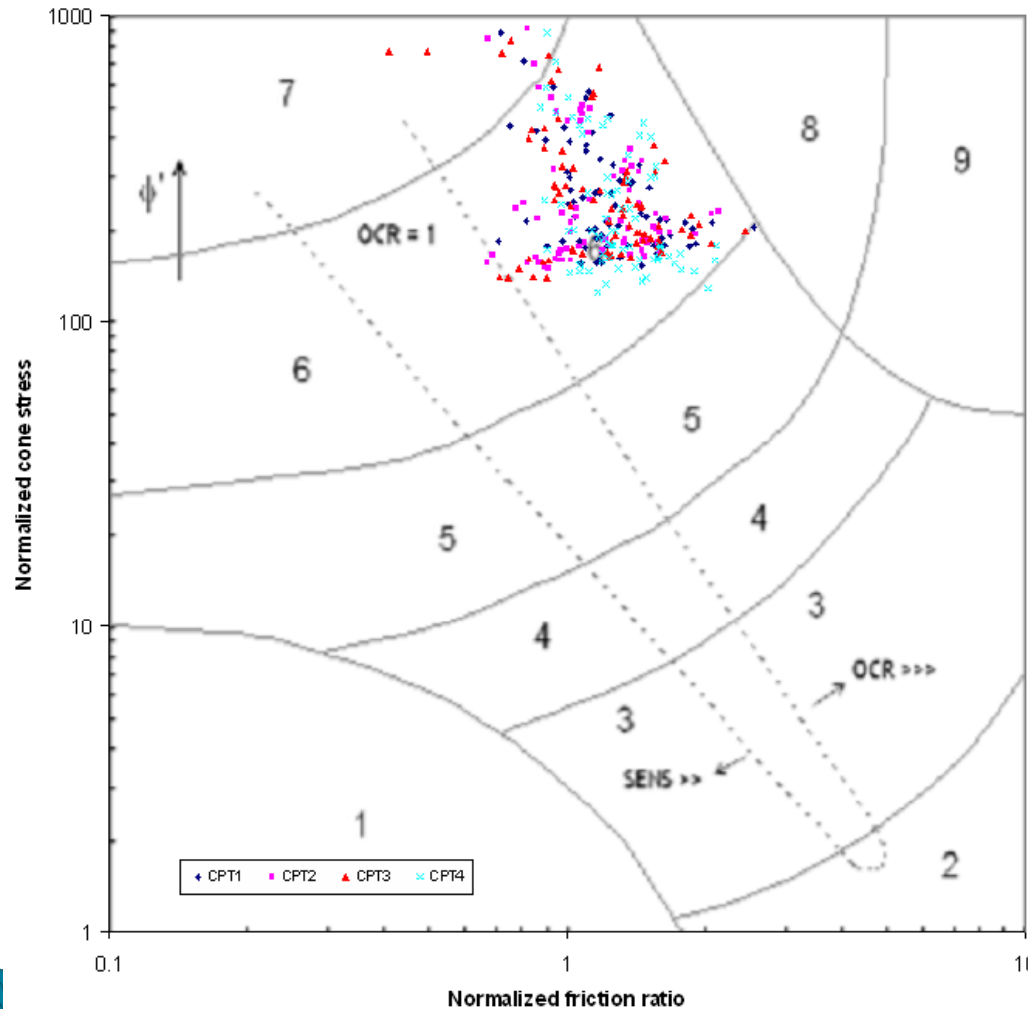
# Problem description



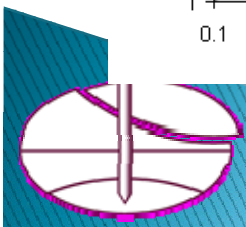
# Results of cone penetration tests (measured values)



# Idealization of the soil (derived values)



1. Sensitive fine-grained soil
2. Organic soils and peat
3. Clays [clay to silty clay]
4. Silt mixtures [silty clay to clayey silt]
5. Sand mixtures [sandy silt to silty sand]
6. Sand [silty sand to clean sand]
7. Sand to gravelly sand
8. Sand – clayey sand to “very stiff” sand
9. Very stiff, fine-grained, overconsolidated or cemented soil



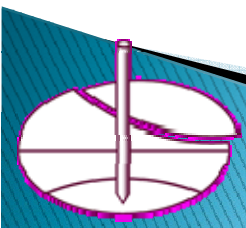
## Derived values

*The Young's modulus of elasticity, for calculating the settlement of spread foundations can be determined by the equation proposed on Annex D of EN 1997-2.*

$$E' = 2.5 * q_c$$

*For the determination of the soil shear resistance angle, from the CPT resistance, it's proposed on Annex D of EN 1997-2:2007 the equation:*

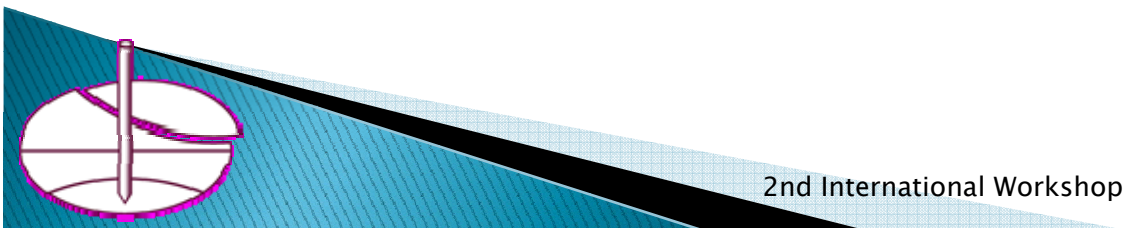
$$\phi' = 13.5 * \log_{10}(q_c) + 23$$



# Soil characterization

There are two main interdependent tasks to be considered in most of geotechnical design problems:

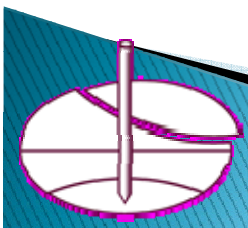
- a geometrical task, where the soil is idealized into a few of well defined and homogeneous layers;
- a subsequent task, where the geomechanical properties of each layer are assigned.



# Characteristic values

EN 1990 defines a characteristic material property as follows:

*“[EN 1990 §4.2(3)] – where a low value of material or product property is unfavourable, the characteristic value should be defined as the 5% fractile value;  
– where a high value of material or product property is unfavourable, the characteristic value should be defined as the 95% fractile value.”*



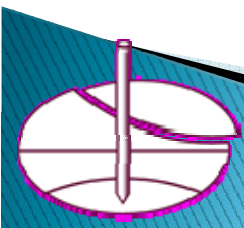


# Characteristic values

*Eurocode 7 redefines the characteristic value as:*

*“[EN 1997–1 §2.4.5.2 (2)P] The characteristic value of a geotechnical parameter shall be selected as a cautious estimate of the value affecting the occurrence of the limit state.”*

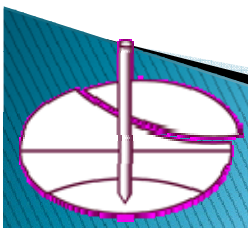
*“[EN 1997–1 §2.4.5.2 (11)] If statistical methods are used, the characteristic value should be derived such the calculated probability of a worse value governing the occurrence of the limit state under consideration is not greater than 5.”*



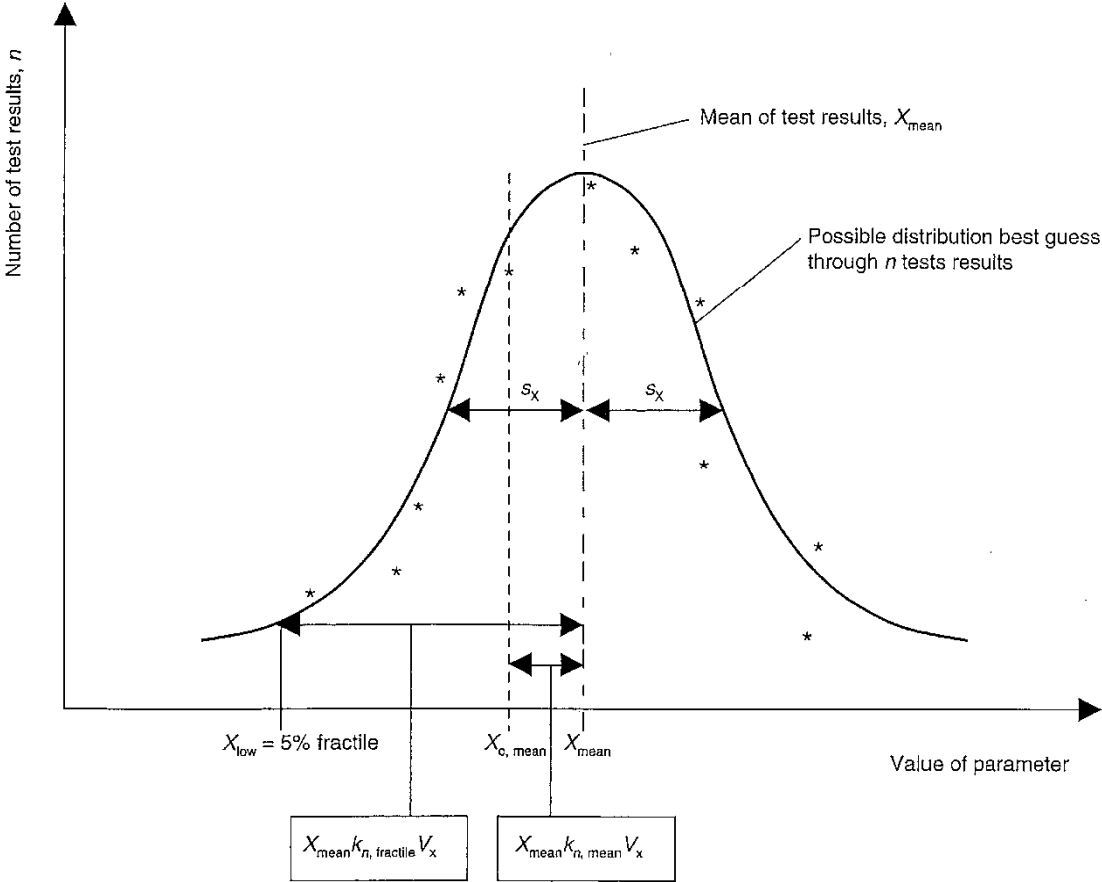
# Characteristic values

*Frank et. al (2004) explains that there are two main aspects to consider when selecting the characteristic value, which are:*

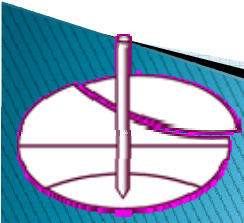
- *the **degree of confidence** in the information (which includes the amount of information on the soil characteristics and the variability of results);*
- *the **soil volume involved** in the limit state considered and the ability of the structure to transfer loads from weak to strong zones of the ground.*



# Characteristic values



Difference between cautious estimate of the mean and of the 5% fractile value (Frank et. al, 2004)

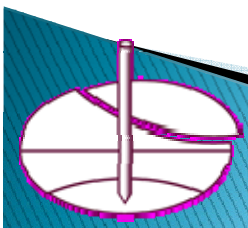


# Characteristic values

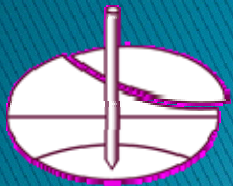
*Schneider (1997) defines the characteristic value as the “best estimate of the unknown statistical mean  $x_m$  of a soil layer”. By this he means that the characteristic value shall be selected with the aim that the probability of a more adverse (mean) value governing the behaviour of the soil and rock in the ground is not greater than 5%.*

*Schneider shows that a suitable equation for the determination of characteristic value for most of soil properties is given by*

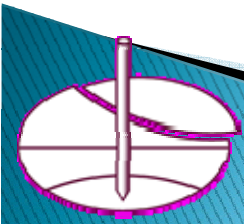
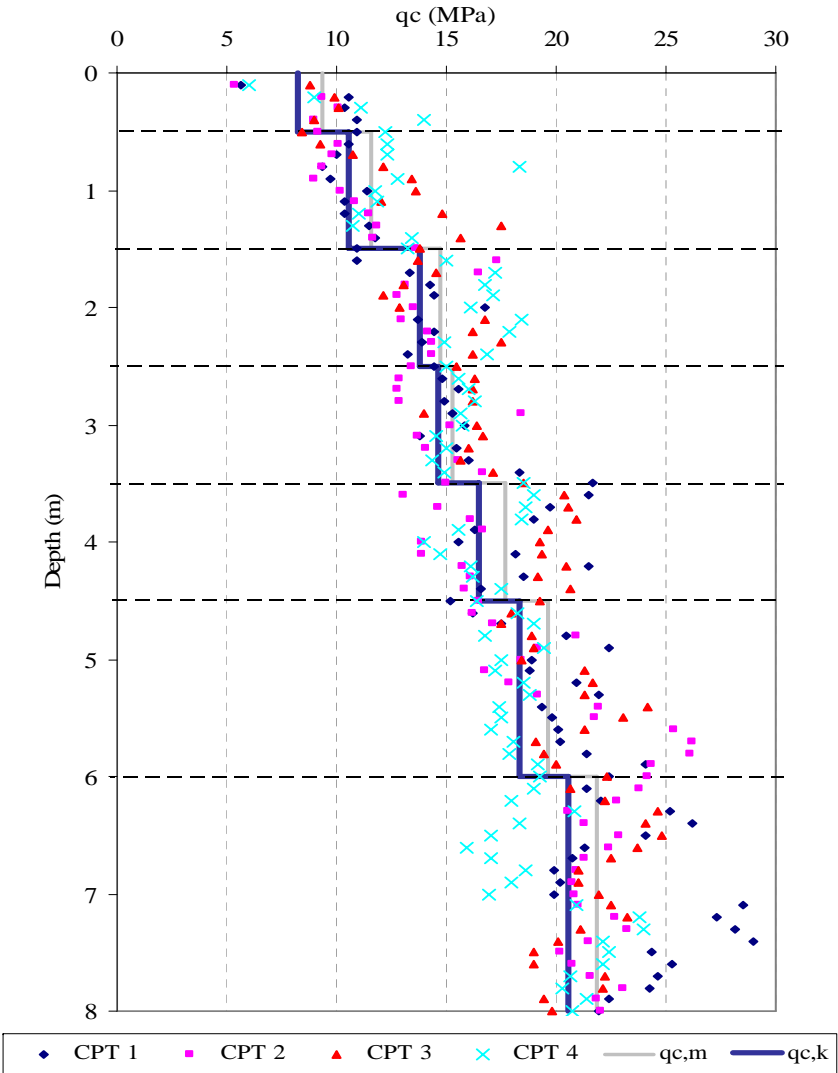
$$x_k = x_m * \left( 1 - \frac{V_x}{2} \right)$$



# Proposed resolution

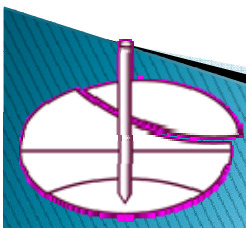


# Characteristic cone resistance

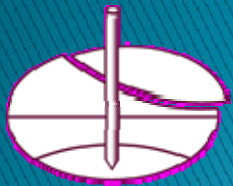


# Characteristic design values

Layer no.	Depth (m)	Mean depth (m)	$q_{c,m}$ (MPa)	$q_{c,k}$ (MPa)	E (MPa)	$\phi$ (°)
1	[0.0; 0.5]	0.25	9.32	8.22	20.6	35.4
2	[0.5; 1.5]	1.00	11.60	10.52	26.3	36.8
3	[1.5; 2.5]	2.00	14.72	13.77	34.4	38.4
4	[2.5; 3.5]	3.00	15.32	14.67	36.7	38.7
5	[3.5; 4.5]	4.00	17.67	16.45	41.1	39.4
6	[4.5; 6.0]	5.25	19.60	18.33	45.8	40.1
7	[6.0; 8.0]	7.00	21.83	20.58	51.4	40.7

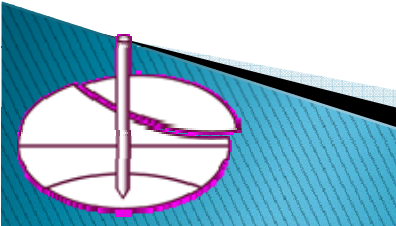
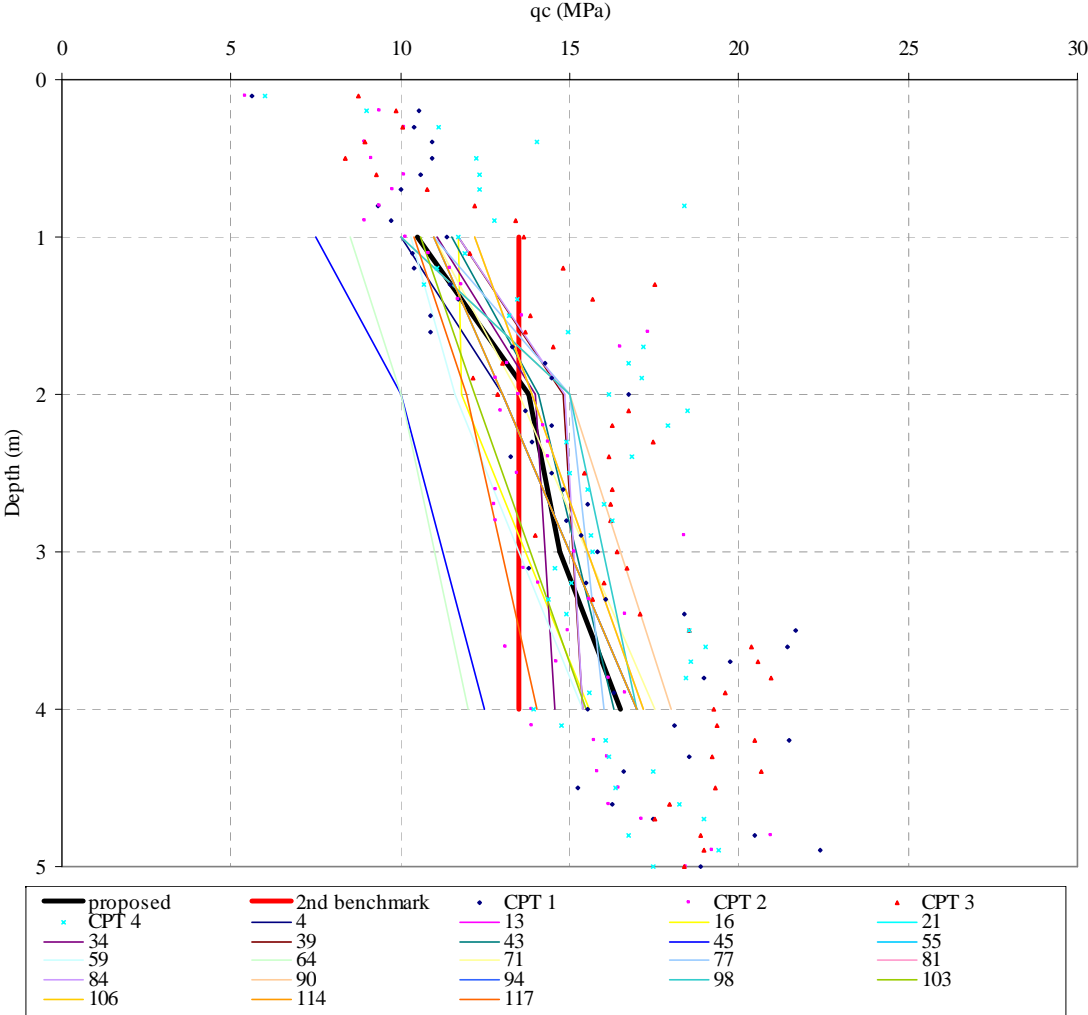


# Participants answers

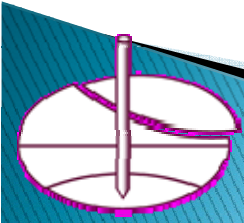
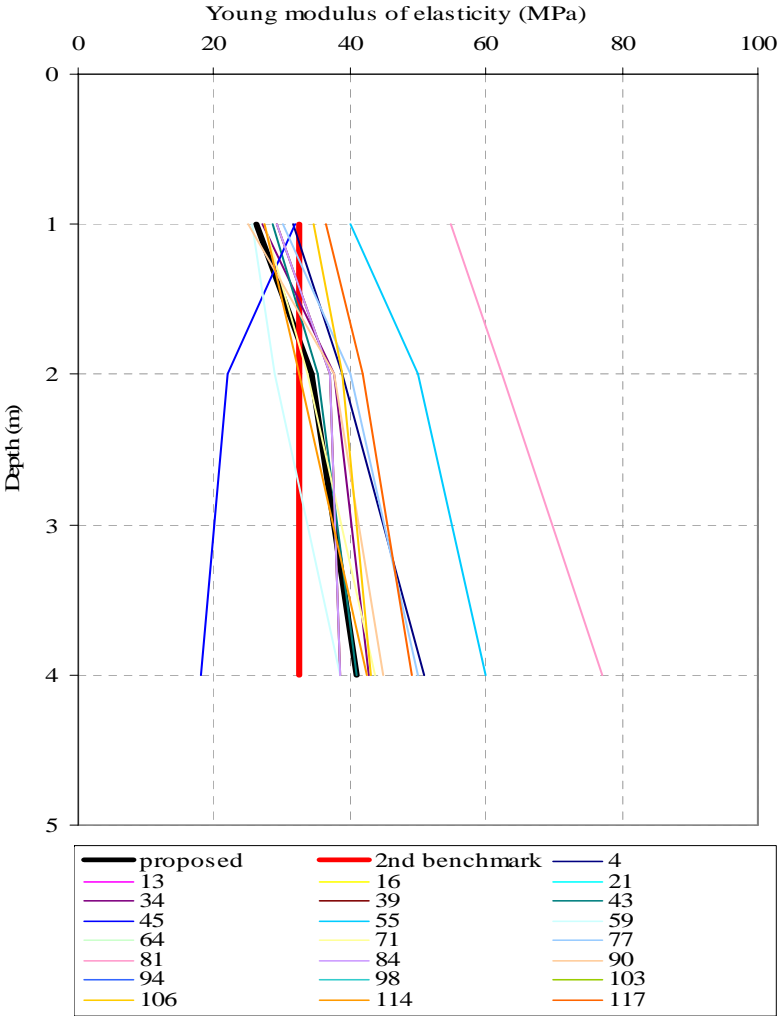




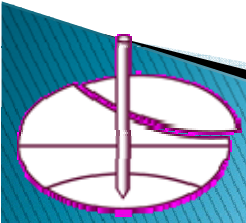
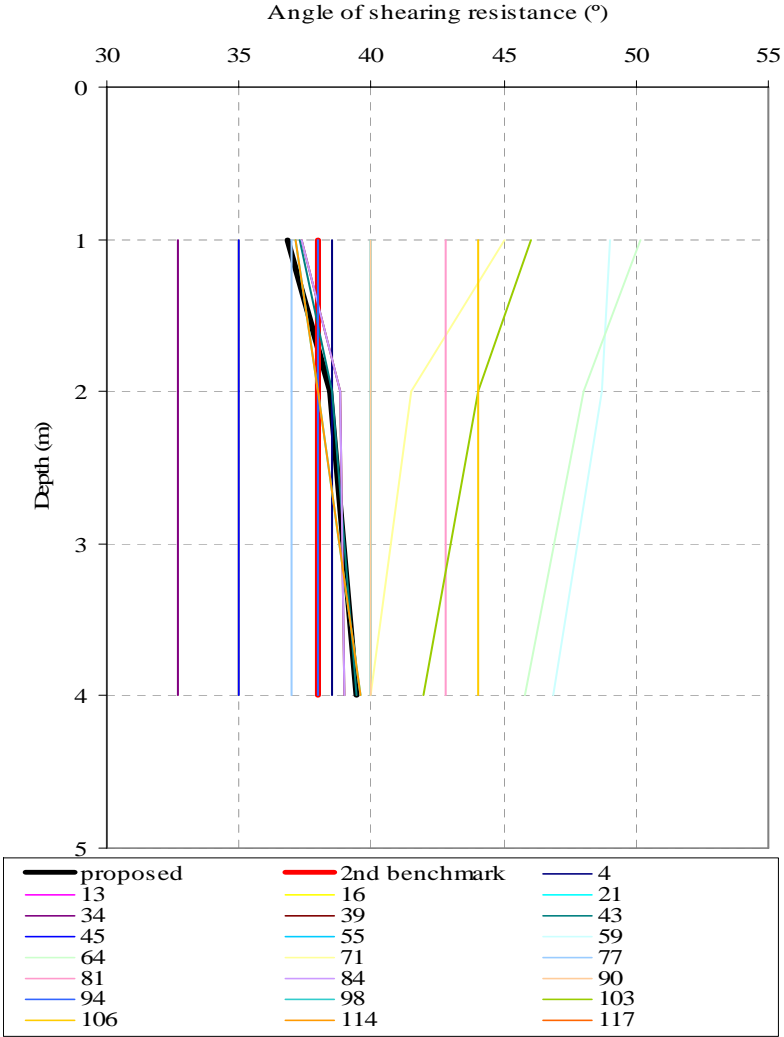
# Characteristic cone resistance



# Characteristic Young modulus of elasticity



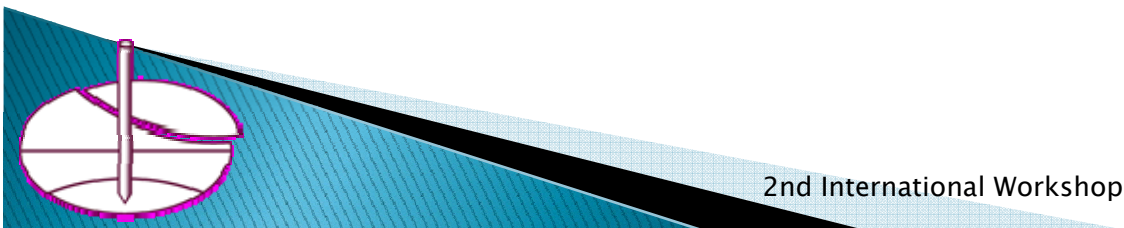
# Characteristic angle of shearing resistance



# Design approaches

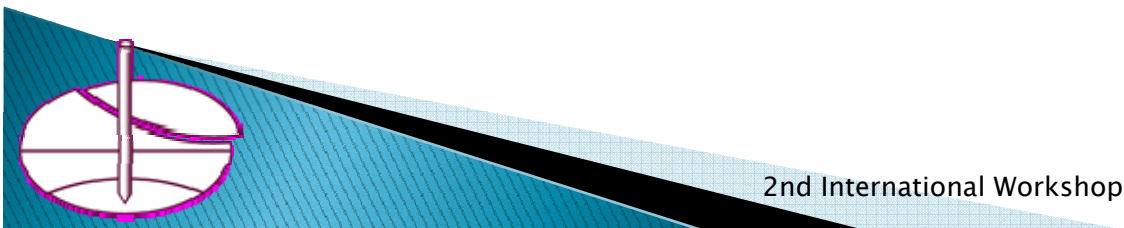
## National choices

DA 1 com 1 and 2	GB, IT, PT
DA 2	GR, FR, DE, PL, IE
DA 3	DK, NL



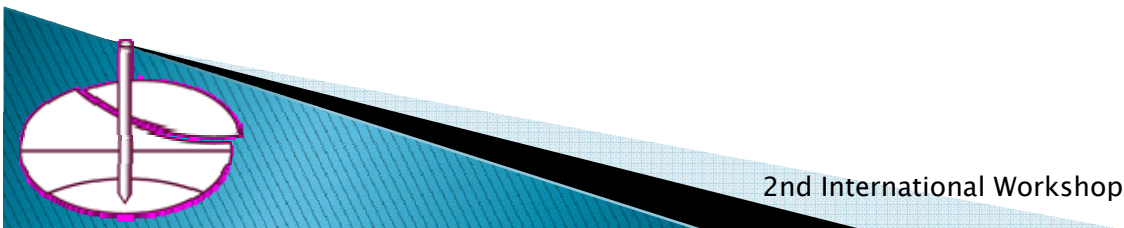
# Method used (ULS)

Annex D	10 answers
National annexes	5 answers
Brinch Hansen	4 answers
Terzaghi	1 answer
Highway bridges (Japan)	1 answer

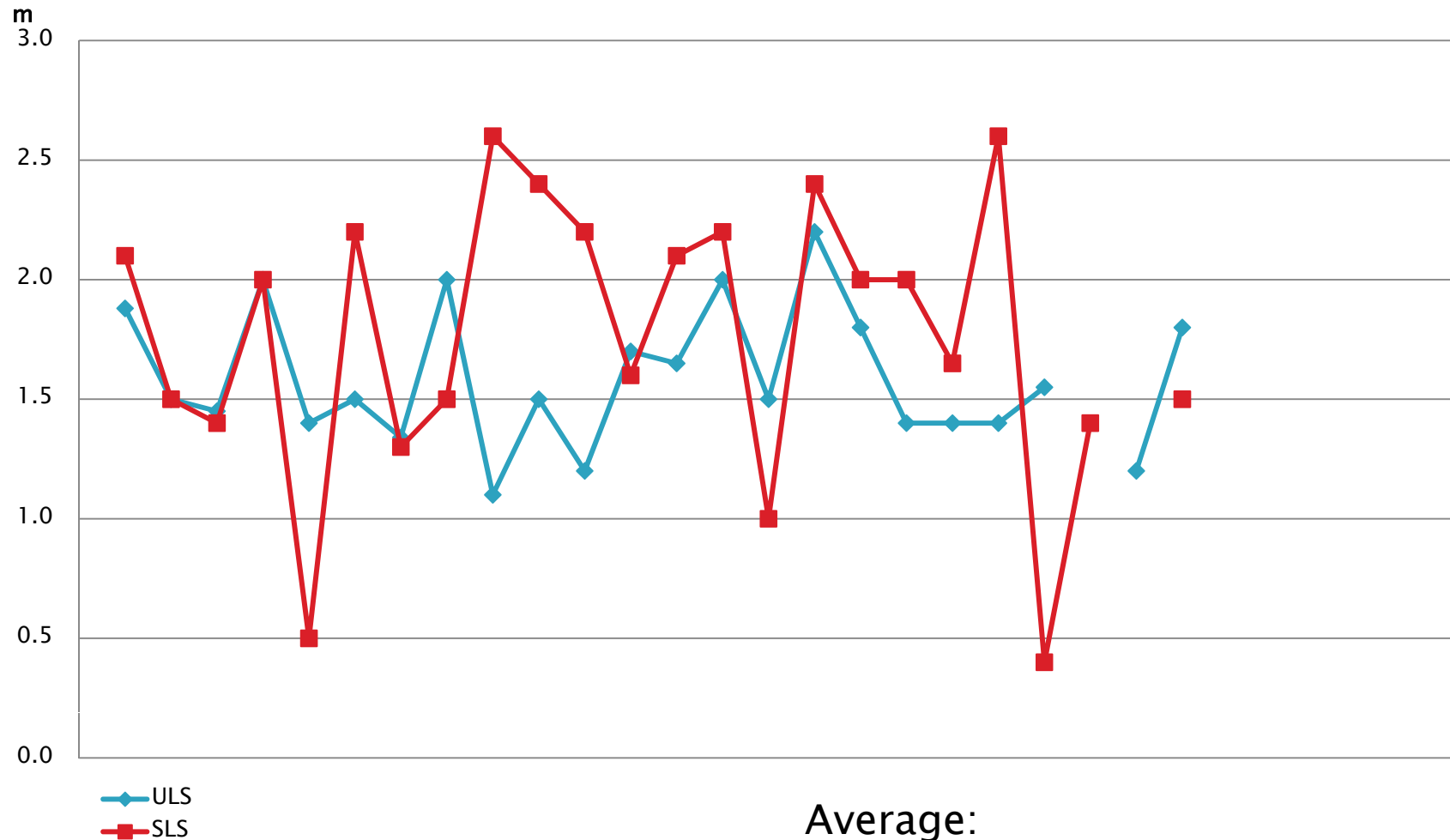


# Method used (SLS)

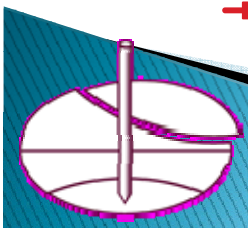
Annex D.3 EN 1997-1	9 answers
Annex F.1 EN 1997-2	5 answers
National annexes and different methods as Schmertmann, Tomlinson, Burland & Bridge	10 answers



# Width of pad (conclusion)



Average:  
ULS 1.6 m and SLS 1.8 m



# Comparison of foundation width and $\tan(\phi)$

